

Large head screw

Carbon steel with white galvanic zinc coating



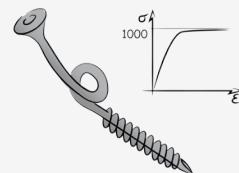
PACKAGING

Box + CE paper + BIT



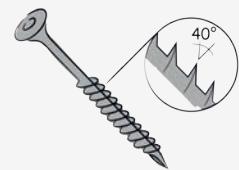
SPECIAL STEEL

Highly ductile (moves with the wood) and resistant steel ($f_{y,k} = 1000 \text{ N/mm}^2$)



SPECIAL THREADING

Asymmetric "umbrella" threading for better wood penetration



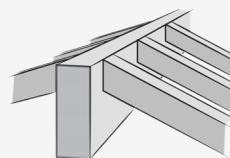
ECO-FRIENDLY

Trivalent Cr³⁺ chrome coating, replacing hexavalent chrome Cr⁶



FIELDS OF USE

Solid-wood, glulam, X-Lam, LVL, wood-based panels. Service classes 1 and 2.





JOINT SEAL

The large head guarantees excellent tensile strength which means the use of additional lateral fastening systems can be avoided



JOINT CLOSURE

The large head offers tight joint closure. The diameter of the head is optimised based on the length of the thread



JOINT STABILITY

The large head guarantees excellent penetration resistance and allows for stabilisation of joints subject to variations in the dimensions of the wood

Applications



Fastening walls at the corners of the structure in X-Lam with excellent joint closure



Fastening walls at the corners of the frame structure with excellent joint closure

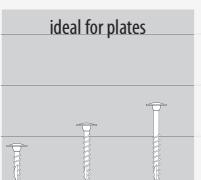
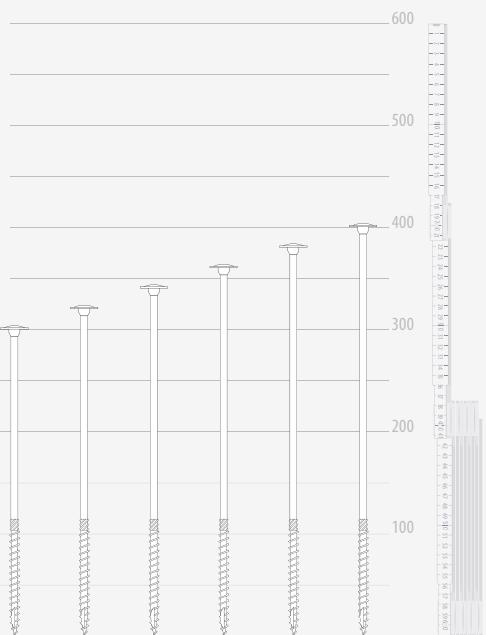


Fastening of plaster fibre panels



Range

The large head offers excellent grip capacity and makes it possible to obtain greater thread length with respect to a screw with a classic head. The size of the head diameter (dk) is calculated according to the thread length to obtain the perfect proportion between head penetration resistance and the thread grip capacity. In this way, the risk of stripping the screw is avoided. 8mm screws with a length between 40 and 80mm are ideal for fastening to steel plates.



ø6
TX30

ø8
TX40

ø10
TX40

Codes and dimensions

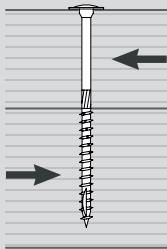
	d_1 [mm]	d_k [mm]	code	L [mm]	b [mm]	A [mm]	pcs/pckg
6 TX30	15,5	6	TBS680	80	50	30	
			TBS6100	100	60	40	
			TBS6120	120	75	45	
			TBS6140	140	75	65	
			TBS6160	160	75	85	
			TBS6180	180	75	105	
			TBS6200	200	75	125	50
			TBS6220	220	100	120	
			TBS6240	240	100	140	
			TBS6260	260	100	160	
			TBS6280	280	100	180	
			TBS6300	300	100	200	
8 TX40	19	8	TBS840	40	32	8	
			TBS860	60	52	10	
			TBS880	80	52	28	
			TBS8100	100	80	20	
			TBS8120	120	80	40	
			TBS8140	140	80	60	
			TBS8160	160	100	60	
			TBS8180	180	100	80	
			TBS8200	200	100	100	
			TBS8220	220	100	120	50
			TBS8240	240	100	140	
			TBS8260	260	100	160	
			TBS8280	280	100	180	
			TBS8300	300	100	200	
			TBS8320	320	100	220	
			TBS8340	340	100	240	
			TBS8360	360	100	260	
			TBS8380	380	100	280	
			TBS8400	400	100	300	
10 TX40	25	10	TBS10160	160	80	80	
			TBS10180	180	80	100	
			TBS10200	200	100	100	
			TBS10220	220	100	120	
			TBS10240	240	100	140	
			TBS10260	260	100	160	
			TBS10280	280	100	180	50
			TBS10300	300	100	200	
			TBS10320	320	100	220	
			TBS10340	340	100	240	
			TBS10360	360	100	260	
			TBS10380	380	100	280	
			TBS10400	400	100	300	

Carpenter statics

ALLOWABLE VALUES
DIN 1052:1988

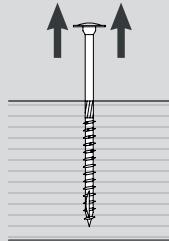
SHEAR V_{adm}

WOOD-WOOD



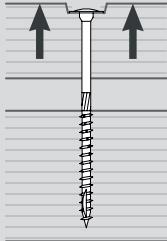
d_1 [mm]	L [mm]	V_{adm}
6	≥ 80	61 kg
8	≥ 120	109 kg
10	≥ 160	170 kg

THREAD WITHDRAWAL N_{adm}



d_1 [mm]	Length L [mm]									
	40	60	80	100	120 - 140	160	180	200	220 - 300	320-400
6	-	-	150 kg	180 kg	225 kg	225 kg	225 kg	225 kg	300 kg	-
8	128 kg	208 kg	208 kg	320 kg	320 kg	400 kg	400 kg	400 kg	400 kg	400 kg
10	-	-	-	-	-	400 kg	400 kg	500 kg	500 kg	500 kg

HEAD PENETRATION N_{adm}



d_1 [mm]	N_{adm}
6	120 kg
8	181 kg
10	281 kg

CALCULATION FORMULAS - SHEAR DIN 1052-2:1988

WOOD-WOOD

$$V_{adm} = \min \{ 0,4 \cdot A \cdot d_1; 1,7 \cdot d_1^2 \}$$

d_1 [mm]
 A [mm]
 V_{adm} [kg]

EXAMPLE WOOD-WOOD

TBS 6 x 160 mm

$d_1 = 6$ mm
 $A = 85$ mm

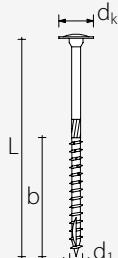
$V_{adm} = \min \{ 0,4 \cdot A \cdot d_1; 1,7 \cdot d_1^2 \}$
 $V_{adm} = \min \{ 0,4 \cdot 85 \cdot 6; 1,7 \cdot 6^2 \} = \min \{ 204 ; 61 \} = 61$ kg

NOTE

- Allowable values in accordance with DIN 1052:1988.
- The allowable shear values are calculated considering a fixing length of $8 d_1$.
- The allowable extraction values are calculated considering the threaded part as being completely inserted into the wood.

Geometry and minimum distances

GEOMETRY AND MECHANICAL CHARACTERISTICS

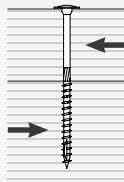


TBS SCREWS

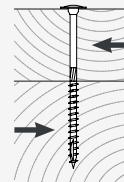
Nominal diameter

	6	8	10
Head diameter d_k [mm]	15,50	19,00	25,00
Tip diameter d_2 [mm]	3,95	5,40	6,40
Shank diameter d_s [mm]	4,30	5,80	7,00
Pre-bored hole diameter d_y [mm]	4,0	5,0	6,0
Characteristic yield moment $M_{y,k}$ [Nm]	9493,7	20057,5	35829,6
Characteristic extraction-resistance parameter $f_{ax,k}$ [N/mm ²]	11,7	11,7	11,7
Characteristic head-penetration parameter $f_{head,k}$ [N/mm ²]	10,5	10,5	10,5
Characteristic tensile strength $f_{tens,k}$ [kN]	11,3	20,1	31,4

MINIMUM DISTANCES FOR SHEAR LOADS



Angle between strength and grain $\alpha = 0^\circ$



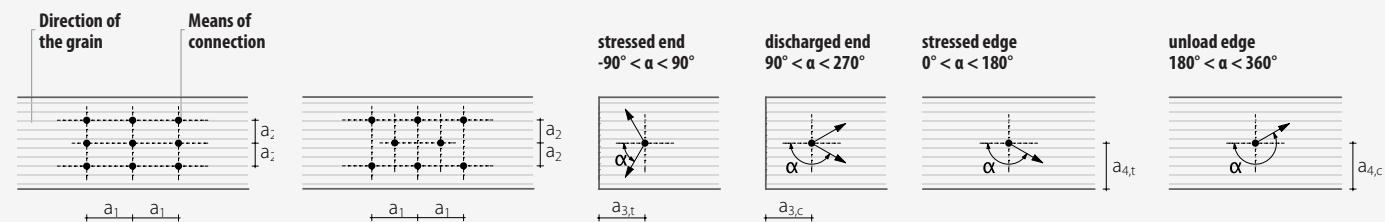
Angle between strength and grain $\alpha = 90^\circ$

SCREWS INSERTED WITH PRE-BORED HOLES

	6	8	10	6	8	10
a_1 [mm]	30	40	50	24	32	40
a_2 [mm]	18	24	30	24	32	40
$a_{3,t}$ [mm]	72	96	120	42	56	70
$a_{3,c}$ [mm]	42	56	70	42	56	70
$a_{4,t}$ [mm]	18	24	30	42	56	70
$a_{4,c}$ [mm]	18	24	30	18	24	30

SCREWS INSERTED WITHOUT PRE-BORED HOLES

	6	8	10	6	8	10
a_1 [mm]	72	96	120	30	40	50
a_2 [mm]	30	40	50	30	40	50
$a_{3,t}$ [mm]	90	120	150	60	80	100
$a_{3,c}$ [mm]	60	80	100	60	80	100
$a_{4,t}$ [mm]	30	40	50	60	80	100
$a_{4,c}$ [mm]	30	40	50	30	40	50



NOTE

- The minimum distances are in accordance with the EN 1995:2008 standard, according to ETA-11/0030, considering a mass density of the wood elements of $\rho_k \leq 420 \text{ kg/m}^3$.
- In the case of OSB-wood joints, the minimum spacings (a_1, a_2) can be multiplied by a coefficient of 0.85.

SHEAR

TRACTION

geometry				wood-wood	panel-wood ⁽¹⁾	thread withdrawal ⁽²⁾	head penetration
d_1 [mm]	L [mm]	b [mm]	A [mm]	$R_{V,k}$ [kN]	$R_{V,k}$ [kN]	$R_{ax,k}$ [kN]	$R_{head,k}$ [kN]
6	80	50	30	2,13	$S_{PAN} = 50 \text{ mm}$	2,12	3,75
	100	60	40	2,33		2,63	4,50
	120	75	45	2,33		2,63	5,62
	140	75	65	2,33		2,63	5,62
	160	75	85	2,33		2,63	5,62
	180	75	105	2,33		2,63	5,62
	200	75	125	2,33		2,63	5,62
	220	100	120	2,33		2,63	7,50
	240	100	140	2,33		2,63	7,50
	260	100	160	2,33		2,63	7,50
8	280	100	180	2,33	$S_{PAN} = 65 \text{ mm}$	2,63	7,50
	300	100	200	2,33		2,63	7,50
	40	32	8	1,07		-	4,05
	60	52	10	1,34		-	4,05
	80	52	28	2,99		2,00	5,20
	100	80	20	2,67		3,19	8,00
	120	80	40	3,38		4,09	8,00
	140	80	60	3,67		4,09	8,00
	160	100	60	3,67		4,09	10,00
	180	100	80	3,67		4,09	10,00
	200	100	100	3,67		4,09	10,00
	220	100	120	3,67		4,09	10,00
	240	100	140	3,67		4,09	10,00
	260	100	160	3,67		4,09	10,00
	280	100	180	3,67		4,09	10,00
10	300	100	200	3,67	$S_{PAN} = 80 \text{ mm}$	4,09	10,00
	320	100	220	3,67		4,09	10,00
	340	100	240	3,67		4,09	10,00
	360	100	260	3,67		4,09	10,00
	380	100	280	3,67		4,09	10,00
	400	100	300	3,67		4,09	10,00
	160	80	80	5,60		6,19	10,00
	180	80	100	5,60		6,19	10,00
	200	100	100	5,60		6,19	12,50
	220	100	120	5,60		6,19	12,50
	240	100	140	5,60		6,19	12,50
	260	100	160	5,60		6,19	12,50
	280	100	180	5,60		6,19	12,50
	300	100	200	5,60		6,19	12,50
	320	100	220	5,60		6,19	12,50
	340	100	240	5,60		6,19	12,50
	360	100	260	5,60		6,19	12,50
	380	100	280	5,60		6,19	12,50
	400	100	300	5,60		6,19	12,50

GENERAL PRINCIPLES

- Characteristic values comply with the EN 1995:2008 standard in accordance with ETA-11/0030.
- Design values are obtained from the following characteristic values: $R_d = \frac{R_k \cdot k_{mod}}{\gamma_m}$
- The coefficients γ_m and k_{mod} should be taken according to the current regulations used for the calculation.
- For the mechanical resistance values and the geometry of the screws, reference was made to ETA-11/0030..
- In the calculations, the density of the wood elements was considered equal to $\rho_e = 380 \text{ kg/m}^3$.
- Characteristic resistances can also be considered as valid for larger densities, for the purposes of safety.
- Values were calculated considering the threaded part as being completely inserted into the wood.
- Sizing and verification of the wooden elements and panels must be done separately.
- The shear characteristic resistances are calculated for screws inserted without pre-bored holes. In the case of screws inserted with pre-bored holes, greater resistance values can be obtained.
- For different calculation methods, the myProject software is available free of charge (www.rothoblaas.com)
- The characteristic resistances were calculated using solid wood or glulam. In the case of joints with X-Lam elements, the resistance values may be different and should be calculated on the basis of the characteristics of the panel and the connection configuration.

NOTE

⁽¹⁾ The shear characteristic resistances are calculated considering a particle board with a S_{PAN} thickness.

⁽²⁾ The axial thread-extraction resistance was calculated considering a 90° angle between the grain and the connector and for a fixing length of b.

Calculation example: rafter - purlin joint with myProject

software
myProject

CONNECTION WOOD-WOOD / SINGLE SHEAR

Download free at www.rothoblaas.com



PROJECT DATA	SCREW SELECTION	CONNECTION GEOMETRY
$F_{v,Rd} = 1,89 \text{ kN}$ Service class = 1 Load duration = short	TBS = 8 x 260 mm Pre-bored hole = no	$t_1 = 160 \text{ mm}$ $\alpha_1 = 0^\circ$ $t_2 = 100 \text{ mm}$ (fixing length in element 2) $\alpha_2 = 90^\circ$

SHEAR RESISTANCE CALCULATION WITH myProject SOFTWARE (EN 1995:2008 e ETA-11/0030)

$d_1 = 8,0 \text{ mm}$
 $f_{h,1,k} = 16,70 \text{ N/mm}^2$
 $f_{h,2,k} = 16,70 \text{ N/mm}^2$
 $\beta = 1,00$
 $M_{y,k} = 20057,5 \text{ Nmm}$

$R_{ax,Rk} = \min \{\text{resistance to thread withdrawal; resistance to head penetration}\} = \min \{R_{ax,Rk}; R_{head,Rk}\} = 4,05 \text{ kN}$
 $R_{ax,Rk} / 4 = 1,01 \text{ kN}$ (cable effect)

$$R_{v,Rk} = 3,67 \text{ kN}$$

$$R_{v,Rd} = \frac{R_{v,Rk} \cdot k_{mod}}{\gamma_m} \geq F_{v,Rd}$$

EN 1995:2008

$$k_{mod} = 0,9$$

$$\gamma_m = 1,3$$

$$R_{v,Rd} = 2,54 \text{ kN} > 1,89 \text{ kN OK}$$

Italia - NTC 2008

$$k_{mod} = 0,9$$

$$\gamma_m = 1,5$$

$$R_{v,Rd} = 2,20 \text{ kN} > 1,89 \text{ kN OK}$$